



Structure Foundation Recommendations

Prepared for: TGS Engineers, Inc. 201 W. Marion Street, Suite 200 Shelby, North Carolina 28150

July 2, 2024





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July 2, 2024

Mr. Jimmy L. Terry, P.E. TGS Engineers, Inc. 201 W. Marion Street, Suite 200 Shelby, North Carolina 28150

WBS ELEMENT:	SF-250208
T.I.P. NO.:	BP6.R021
I.D. NO.:	BP6.R021
COUNTY:	Cumberland
DESCRIPTION:	Replace Structure No. 250208 on SR 2046 (Avery Road) over Turnbull Creek
SUBJECT:	Structure Foundation Recommendations

Dear Mr. Terry,

Carolinas Geotechnical Group, PLLC (CG2) has completed the Structure Foundation Recommendations for Structure No. 250208 on SR 2046 (Avery Road) over Turnbull Creek in Cumberland County, North Carolina. Foundation Recommendations Notes on Plans and Comments, the prepared NCDOT Structure Inventory Report, and supporting calculations are presented below and attached.

CULVERT -L- 14+27.00

FOUNDATION RECOMMENDATION NOTES ON PLANS:

- 1. INSTALL PIPE CULVERT IN ACCORDANCE WITH SECTION 300 OF THE STANDARD SPECIFICATIONS.
- 2. EXCAVATE 12 INCHES BELOW THE BOTTOM OF THE CULVERT AND REPLACE WITH FOUNDATION CONDITIONING MATERIAL IN ACCORDANCE WITH SECTION 414 OF THE STANDARD SPECIFICATIONS. FOUNDATION CONDITIONING MATERIAL SHOULD CONSIST OF SELECT MATERIAL CLASS V OR VI FOR PIPE CULVERTS.
- 3. IF REQUIRED, UNDERCUT LOOSE SOILS THAT MAY BE ENCOUNTERED BENEATH THE BOTTOM OF THE FOUNDATION CONDITIONING MATERIAL. BACKFILL UNDERCUT AREAS WITH FOUNDATION CONDITIONING MATERIAL.

FOUNDATION RECOMMENDATION COMMENTS:

- 1. The invert elevation at the centerline of the aluminum structural plate pipe arch is 86.5 feet.
- 2. We anticipate groundwater to impact construction.
- 3. We recommend a quantity of 110 tons of foundation conditioning material (Class V or VI).



Structure Foundation Recommendations

Structure No. 208 on SR 2046 (Avery Road) over Turnbull Creek

Cumberland County, North Carolina

- 4. We do not anticipate settlement to be a concern.
- 5. Place Select Material Class V or VI when backfilling in water.

CLOSING

Please do not hesitate to contact us if you have any questions regarding this report or if you need additional services.

Sincerely, Carolinas Geotechnical Group, PLLC



F7633FB568FE4D7... Kelly N. de Montbrun, P.E. Senior Project Engineer



DocuSigned by:

Michael J. Walko Michaef YAWAIRO, P.E. Principal Engineer

ATTACHMENTS:

Structure Subsurface Investigation Report (Prepared by CG2) Culvert Survey & Hydraulic Design Report Supporting Calculations

ATTACHMENTS



CULVERT -L- 14+27.00 - STRUCTURE SUBSURFACE INVESTIGATION REPORT (PREPARED BY CG2)



CONTENTS

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SHEET NO. R02 P6.

8

REFERENCE

DESCRIPTION TITLE SHEET LEGEND (SOIL & ROCK) SITE PLAN BORELOGS

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY CUMBERLAND

PROJECT DESCRIPTION REPLACE STRUCTURE NO. 208 ON SR 2046 (AVERY ROAD) OVER TURNBULL

CREEK

SITE DESCRIPTION 14+27 -L-

250208 5 PROIEC

STATE N.C

STATE PROJECT REFERENCE NO. **BP6.R021**

NO

SHEETS

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CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALFICH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENCINEERING UNIT AT (1991) 707-8650. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CENERAL SOL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UN-PLACED TEST DATA CAN BE RELIED ON ONLY TO THE DECREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOLL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOLL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIODER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBJURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR BIDING DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPHION OF THE DEPARTMENT AS TO THE TYPE OF MATEMALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFRIM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FOM THE ACULAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES: I. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE. 2.

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INVESTIGATED BY	
	PERSONNEL P. PERRY, EIT

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

07/02/2024

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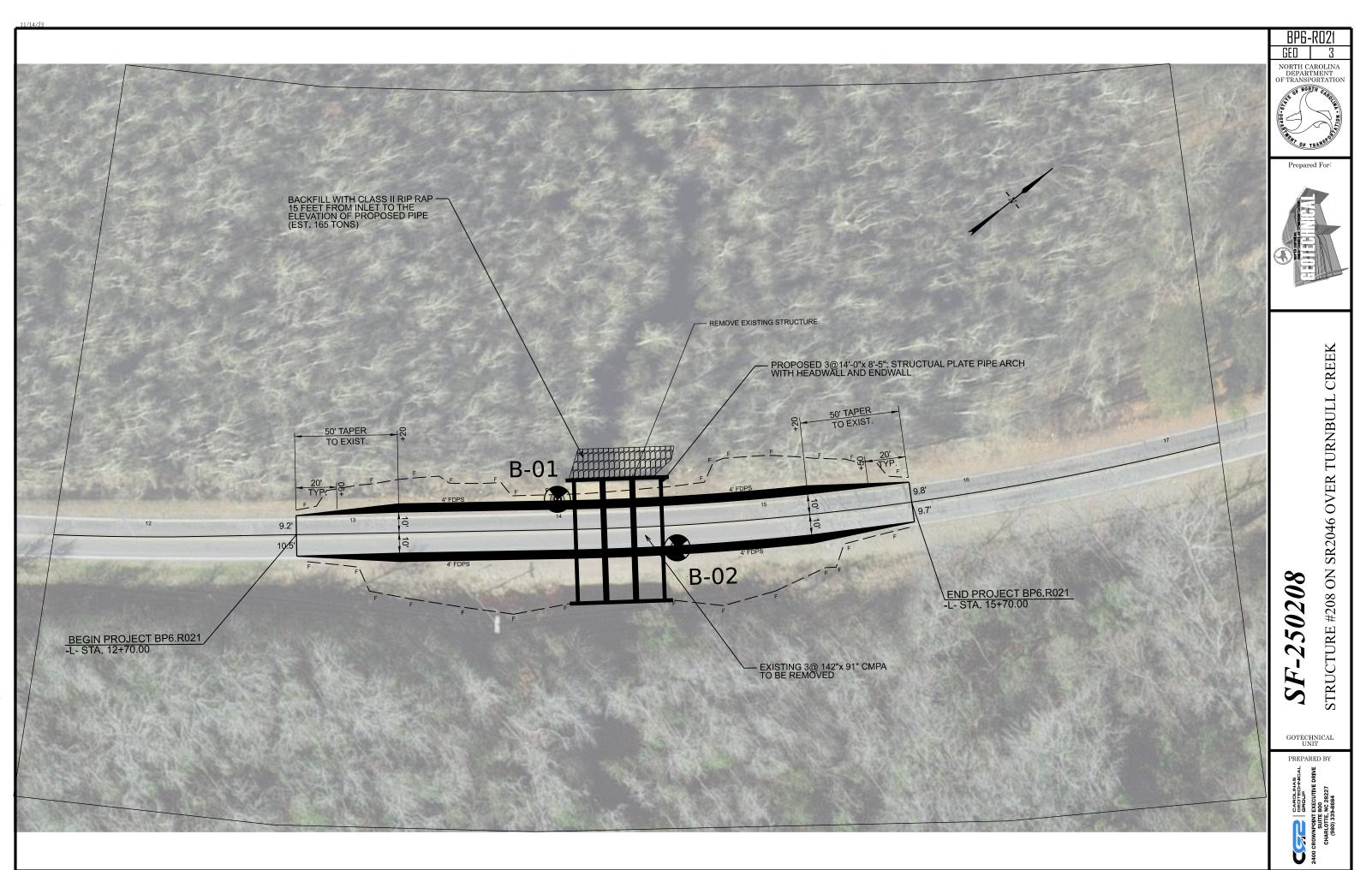
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPT	ION	GRADATION	ROCK DESCRIPTION						
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, D BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER A ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICA'	ND YIELD LESS THAN 100 BLOWS PER FOOT 7 206, ASTM DI586), SOIL CLASSIFICATION 6 GENERALLY INCLUDE THE FOLLOWING; 710N, AND OTHER PERTINENT FACTORS SUCH	<u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTE ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK REPRESENTED BY A ZONE OF WEATHERED ROCK.						
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTU VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FIN	JRE, PLASTICITY, ETC. FOR EXAMPLE, IE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:						
SOIL LEGEND AND AASHTO		ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED, MINERALOGICAL COMPOSITION	ROCK (WR)						
CLASS. (≤ 35% PASSING 200) (> 35% P	Y MATERIALS ASSING ≠200) A-6 A-7 A-1, A-2 A-4, A-5	MINERAL NAMES SUCH AS QUARTZ, FELOSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC RC WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE IN GNEISS, GABBRO, SCHIST, ETC.						
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7	A-6 A-7 A-1, A-2 A-4, A-5 A-7-5 A-3 A-6, A-7 A-7_6 A-3	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTA ROCK (NCR) FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTA SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL						
SYMBOL 0000 0000000000000000000000000000000		SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ET COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT SEDIMENTARY ROCK STREAM SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDS						
% PASSING *10 50 MX	GRANULAR SILT- CLAY MUCK,	PERCENTAGE OF MATERIAL	CCP) SHELL BEDS, ETC.						
*40 30 MX 50 MX 51 MN *200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 M	SOILS COLC PEAT	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK						
MATERIAL PASSING *40 LL — — 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN PI 6 MX NP 18 MX 11 MN 11 MN 18 MX 18 M	X 11 MN 11 MN LITTLE UR HIGHLY	TRACE OF ORGANIC MATTER 2 3% 3 5% TRACE 1 10% LITTLE ORGANIC MATTER 3 -5% 5 12% LITTLE 08 20% MODERATELY ORGANIC 5 10% 12 -20% SOME 20 -35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	HAMMER IF CRYSTALLINE. VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY C (V SLI) CRYSTALLS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER H OF A CRYSTALLINE NATURE.						
GROUP INDEX 0 0 0 4 MX 8 MX 12 ML USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY SILTY </td <td>X I6 MX NO MX ANDUNTS OF ORGANIC CLAYEY MATTER SOILS</td> <td>GROUND WATER</td> <td>SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO RC (SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONA CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMEN</td>	X I6 MX NO MX ANDUNTS OF ORGANIC CLAYEY MATTER SOILS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO RC (SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONA CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMEN						
MATERIALS SAND	TO POOR FAIR TO POOR UNSUITABLE	▼ STATIC WATER LEVEL AFTER <u>24</u> HOURS ∑PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA ○●●●●●●●●●●●●●●●●●●●●●●●●●●●●●●●●●●●●	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECT (MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CL4 DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH WITH FRESH ROCK.						
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7			MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL F						
	NSENESS F STANDARD RANGE OF UNCONFINED	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE L (MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND						
PRIMARY SOIL TYPE COMPACTNESS OR PENETRATIC CONSISTENCY (N-1) CENEDALLY VERY LOOSE	IN RESISTENCE COMPRESSIVE STRENGTH VALUE) (TONS/FT ²) < 4 TO 10	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL SOIL SYMBOL SOIL SYMBOL SOIL SYMBOL SOIL SYMBOL SOIL SYMBOL SOIL SYMBOL SOIL SYMBOL SOIL SYMBOL	IF TESTED, WOULD YIELD SPT REFUSAL SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND E (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS (TO SOME EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.						
GRANULAR MEDIUM DENSE 10 MATERIAL DENSE 30 (NON-COHESIVE) DENSE 30	TO 30 N/A TO 50 - 50	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS AF SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OI						
GENERALLYSOFT2SILT-CLAYMEDIUM STIFF4MATERIALSTIFF8	< 2 < 0.25 TO 4 0.25 TO 0.5 TO 8 0.5 TO 1.0 TO 15 1 TO 2 TO 30 2 TO 4	INFERRED SOIL BOUNDARY O CORE BORING SOUNDING ROD INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE INFERRED ROCK LINE MONITORING WELL O SOUNDING ROD TEST BORING WITH CORE SOUNDARY A PIEZOMETER SOIN SOUNDARY A PIEZOMETER SOIN SOUNDARY A SOIN SOUNDARY SOIN SOUNDARY A SOIN SOUNDARY A SOIN SOUNDARY SOIN SOUNDARY SOIN SOUNDARY A SOIN SOUNDARY SOU	 IV SEV.) REMAINING. SAPPOLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESED, WOULD YIELD SPT N V</u> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS ALSO AN EXAMPLE. 						
HARD >	· 30 > 4		ROCK HARDNESS						
		RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMEN						
U.S. STD. SIEVE SIZE 4 10 40 OPENING (MM) 4.76 2.00 0.42 COARSE COARSE	60 200 270 0.25 0.075 0.053	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER B TO DETACH HAND SPECIMEN.						
BOULDER (BLDR,) COBBLE (COB,) GRAVEL (GR,) COMBL SAND (CSE, SD,) GRAIN MM 305 75 2.0	SAND (F SD.) SILT (SL.) CLAY (CL.) 0.25 0.05 0.005	UNDERCUT ACCEPTABLE DEGRADABLE ROCK EMIDIMMENT OF BHLKFILL ABBREVIATIONS AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DI HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE D BY MODERATE BLOWS.						
SIZE IN. 12 3 SOIL MOISTURE - CORRELA		BT - BORING TERMINATED MICA: MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY γ - UNIT WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC γ - DRY UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE C HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD POINT OF A GEOLOGIST'S PICK.						
SOIL MOISTURE SCALE FIELD MOISTURE (ATTERBERG LIMITS) DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSURMETER TEST <u>SAMPLE ABBREVIATIONS</u> DPT - DVNAMIC PENETRATION TEST SAP. SAPROLITIC S - BULK	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POIN PIECES CAN BE BROKEN BY FINGER PRESSURE.						
- SATURATED - (SAT.) PLASTIC	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SIL T, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SL1 SLIGHTLY RS - ROCK	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCH FINGERRAIL.						
BANGE < - WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS w - MDISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING						
(P) PL PLASTIC LIMIT	SOLID; AT OR NEAR OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO EQUIPMENT USED ON SUBJECT PROJECT DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	TERM SPACING TERM VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED WIDE 3 TO 10 FEET THICKLY BEDDED						
SL SHRINKAGE LIMIT - DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	CME-45C CLAY BITS AUTOMATIC MANUAL	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0. CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0. VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.00 THINLY LAMINATED 0.00						
PLASTICITY		СИЛЕ -55 СОЛЕ -52261 СОЛЕ -52	INDURATION						
NON PLASTIC PLASTICITY INDEX NON PLASTIC 0-5 SLIGHTLY PLASTIC 6-15		CME-550 HARD FACED FINGER BITS	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HE RUBBING WITH FINGER FREES NUMEROUS GRAINS; FRIABLE CENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.						
MODERATELY PLASTIC 16-25 HIGHLY PLASTIC 26 OR MORE	меріим нісн	X CASING W/ ADVANCER POST HOLE DIGGER PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH ST BREAKS EASILY WHEN HIT WITH HAMMER.						
COLOR		X MOBILE B-29	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL DIFFICULT TO BREAK WITH HAMMER.						
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIO MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. AR		Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit Image: Core bit	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLI SAMPLE BREAKS ACROSS GRAINS.						

PROJECT REFERENCE NO. BP6.R021

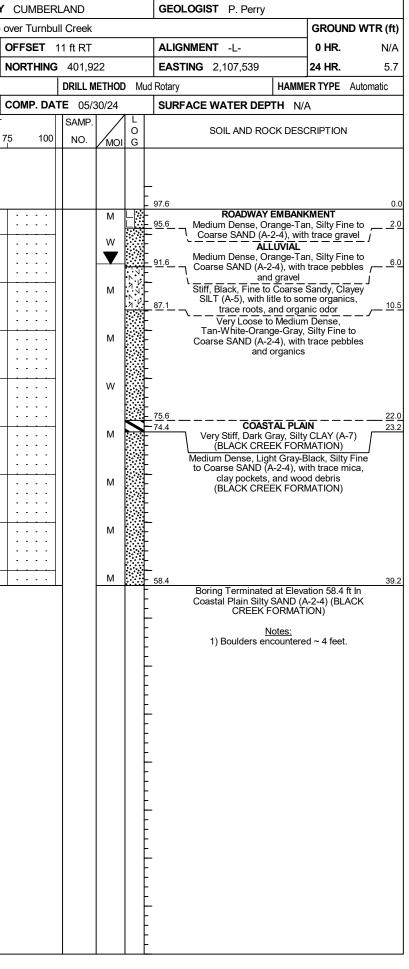
ED. AN INFERRED	
) SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
1 FOOT PER 60 IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.
	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
T N VALUES >	ANOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
OCK THAT ICLUDES GRANITE,	ATTESTAN - UNDUN WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
AL PLAIN IF TESTED. C.	$\underline{\text{COLLUVIUM}}$ - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
MAY NOT YIELD STONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
RINGS UNDER	<u>DIKE</u> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
COATINGS IF OPEN.	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
AMMER BLOWS IF	<u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
DCK UP TO AL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
R BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
S. IN AY. ROCK HAS H AS COMPARED	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
1 HO CUMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
FELDSPARS DULL OSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
EVIDENT BUT	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
ARE KAOLINIZED	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
RE DISCERNIBLE	USUALLY INDICATES POUR AERATION AND LACK OF GOUD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
IF STRONG ROCK T ONLY MINOR	OF AN INTERVENING IMPERVIOUS STRATUM.
VALUES < 100 BPF	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
IN SMALL AND S. SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
IS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
EEP CAN BE DETACHED	<u>SLICKENSIDE</u> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
OR PICK POINT. BLOWS OF THE	STANDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPI) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB.HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
I FRAGMENTS NT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
HED READILY BY	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
THICKNESS	_BENCH_MARK: BM-1: N:401986, E: 2107536 (-LSTA. 15+07.2, 25.8' LT)
4 FEET 1.5 - 4 FEET	ELEVATION: 95.40 FEET
16 - 1.5 FEET	NOTES:
03 - 0.16 FEET 08 - 0.03 FEET < 0.008 FEET	ROADWAY DESIGN AND SURVEY FILES PROVIDED BY TGS.
EAT, PRESSURE, ETC.	
TEEL PROBE;	
PROBE:	
E;	
- ,	DATE: 8-15-14



GEOTECHNICAL BORING REPORT BORE LOG

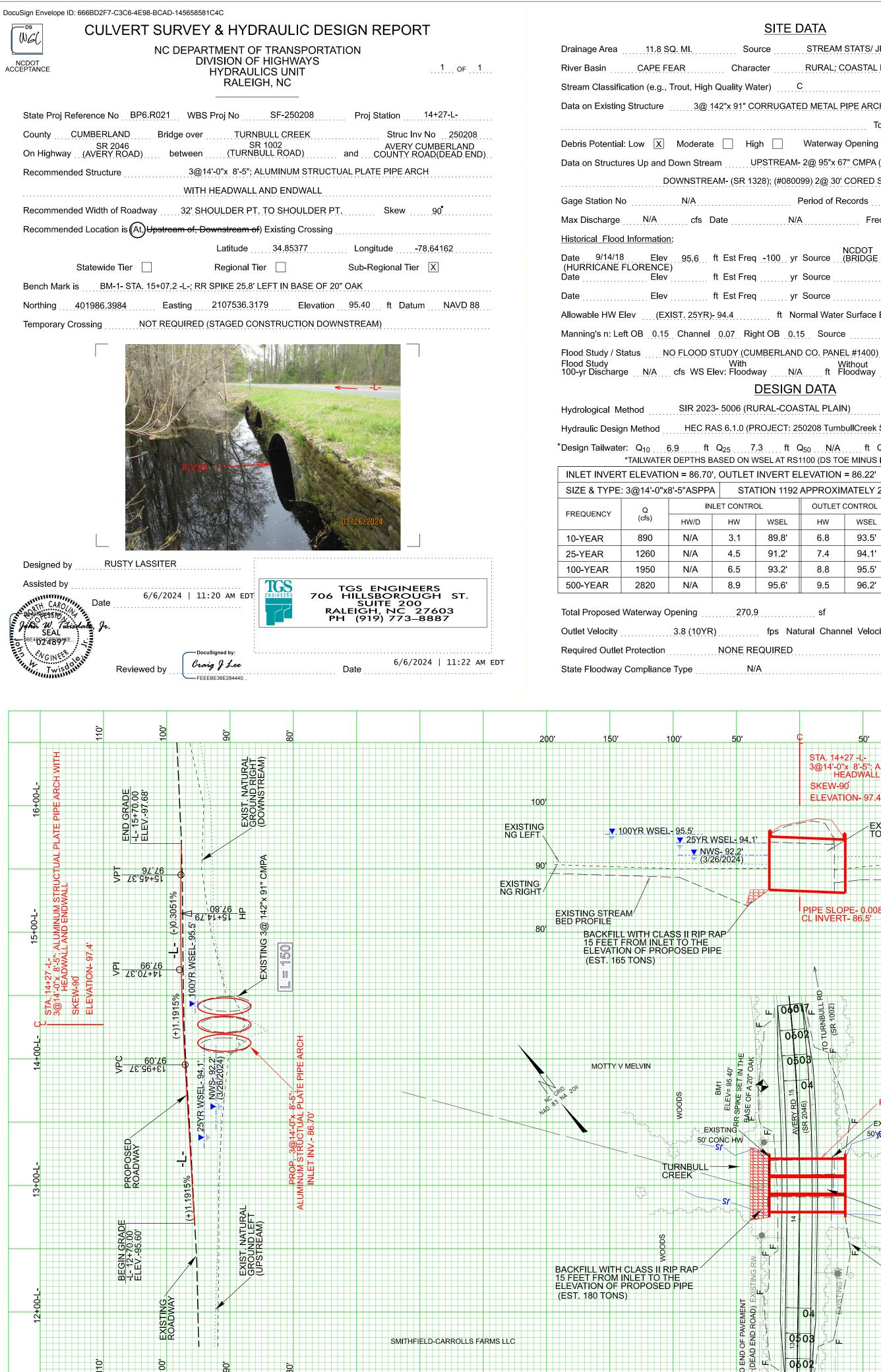
WPC	0F 0F	0200			- 1 -	TIP BP		1	00										Dor				MIDO	SF-25	0200			1-	IP BP6	DOOA		COUN	
	SF-25		Ron	lace P				1 2046 (<i>I</i>								GE		i IST P	. Perry		GROUND					I Ron	laco Pi					very Roa	
	NG NO.		Кер			STATIO					OFFSE					A1		ENT -L			0 HR.	N/A		NG NO.				<u> </u>			,		
	AR ELE		3 2 ft					H 38.8	ft		NORTH			201				2,107			24 HR.	4.6		AR EL							39.2 f	' +	
				E CC				6 04/09/20								Mud Rota		2,10	7,404		RTYPE Au										04/09/202		
	ER M			E 00				06/17/			COMP.									TH N/A		lomalic					E 00				06/17/2		C
		DEPTH	1	DW CC				BLOWS					SAMP.		ΛL					-				DRIVE		1	ow co					PER FO	
(ft)	ELEV (ft)	(ft)	·	0.5ft		0	2		50		75 1	00	NO.	м	0 0 G	ELE	/. (ft)	SOIL	AND RO	CK DESC	RIPTION	DEPTH (ft)	ELEV (ft)	ELEV (ft)	(ft)	0.5ft			0	25		50	75
100																							100										
100	-	-														E							100	97.6	+ - 0.0								
95	96.3	0.0	6	5	5		10					·		M		96.3					MENT , Silty Fine to	0.0	95	94.9	2.7	8	7	8		15	· · · · ·		•
	94.0 _	2.3	2	WOH	1	- <u> - !</u> • 1· ·	 	· · · ·	- - -		 	- -			,	<u>- 54.5</u>		arse SA	ND (A-2 asphalt	-4), with t , and grav	race organics				ł	2	3	10		13 ⁻	· · · · ·	· · · ·	•
90	- 89.0	7.3					•••									89.8		ry Loose	e. Tan-G	LUVIAL ray-Brown	n, Silty, Claye 4), with trace	y <u>6.5</u>	90	89.9	7.7	30	9	1					
	-	_	WOH	WOH	WOF	¶ € 0 1	· · · ·	· · · · · ·	 		· · · · · · ·			W	л л Л	85.8	''	l /ery Soft	pebbles t, Brown-	and organ Black, Fin	nics ne to Coarse	i l			Ŧ					· ·	· · · · ·		
85	84.0	12.3	2	4	7		•••		· · · ·	•••	· · · ·			w				organi	ics, wood	debris, a	, with trace and gravel , Silty Fine to	i l	85	84.9	<u> 12.7</u>	15	9	9		<u>.</u> •••18	· · · · ·	· · · · ·	· ·
80	-						11 •	· · · · · · ·	· · · · · ·		· · · · · · ·	-				<u>80.8</u>	C	Coarse S	Sand (a-	2-4), with	trace gravel	<u> </u>	80	79.9	+ + 177					/ . 	· · · · · · · · · ·		
	79.0	17.3	2	3	6	- - <u>- 1</u> . - . . ● .	· · 9 · ·	· · · ·	 		· · · · · · ·	- - -		м			Lo Fi	ine to Co	/ledium E oarse SA	ND (A-2-	ay-Black, Silt 4), with trace	у			+ 17.7 + +	2	WOH	1			· · · · ·	· · · ·	:
75	-					- 4					· · · ·									d clay sea EK FORM			75	74.9	22.7						· · · · ·		:
		22.3	4	6	9		15	· · · · · · ·	· · · · · ·	•••	· · · · · · ·	• •		м											ŧ	4	9	11		• • 20 • • • • • • • • • • • • • • • • • • •	· · · · ·	· · · · · · ·	
70	69.0	27.3					- - - - -	· · · ·	· · ·		· · · ·	• •											70	69.9	27.7	7	11	16		· · · \		· · · ·	
	-		4	8	10			· · · ·	· · ·		· · · · · · ·	-		M		<u>66.3</u>						<u> 30.0</u>			ł						21 · · · · ·	 	•
65	64.0	32.3	4	5	7		12 ·					-		м			St	(A-7), v	with trace	organics	y, Silty CLAY and mica MATION)		65	64.9	32.7	6	9	13					
60	-	F					~``				· · · ·	-				60.8	 Ha	ind Grav	-Black-F	Rown Fir	e Sandy, Sllt	35.5	60	59.9							· · · · ·		.
	59.0	37.3	7	16	25		· · · ·		 1		· · · · · · ·			w		57.5		LAY (A-	6), with l ce mica a	ittle to so and organ EK FORM	me organics, ic odor	38.8				8	11	18			29		·
	-	F F														Ē		oring Te	erminate	d at Eleva	tion 57.5 ft In CLAY (A-6)			-	ŧ								
	-															Ę		(BLA	CK CRE	EKFORN	MATION)			•	ŧ ŧ								
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SHEET 4



CULVERT SURVEY & HYDRAULIC DESIGN REPORT





INFORMATION TO BE SHOWN ON PLANS SITE DATA HYDRAULIC DATA STREAM STATS/ JEROME QUAD = <u>1260</u> C.F.S. DESIGN DISCHARGE = <u>25</u> yrs. FREQUENCY OF DESIGN FLOOD RURAL; COASTAL PLAIN (REGION 4) = <u>94.1'</u> DESIGN HIGH WATER ELEVATION = <u>11.8</u> sq. мі. DRAINAGE AREA С = <u>1940</u> c.f.s. BASIC DISCHARGE (Q100) 3@ 142"x 91" CORRUGATED METAL PIPE ARCHES = <u>95.5'</u> BASIC HIGH WATER ELEVATION OVERTOPPING FLOOD DATA Total Waterway Opening 213 ft² = <u>1710</u> c.f.s. OVERTOPPING DISCHARGE Debris Potential: Low X Moderate High Waterway Opening Below 100-yr WS Elev 213 ft² REQUENCY OF OVERRTOPPING FLOOD = -100 yrs. UPSTREAM- 2@ 95"x 67" CMPA (SQUATTING BEAR DRIVE) VERTOPPING FLOOD ELEVATION = <u>95.2'</u> * *SHOULDER POINT RIGHT OF 10+84 -L-DOWNSTREAM- (SR 1328); (#080099) 2@ 30' CORED SLAB BRIDGE ON H-PILES (OAL-60') WS EL. Taken @ River Station 1192 Period of Records N/A ADDITIONAL INFORMATION AND COMPUTATIONS N/A N/A Frequency NCDOT Elev <u>95.6</u> ft Est Freq <u>-100</u> yr Source (BRIDGE MAINTENANCE) Period of DA-11.8SQ. MI. RURAL; COASTAL PLAIN Knowledge 25 yrs Period of yr Source Knowledge vrs Period of yr Source Knowledge vrs RURAL COASTAL PLAIN SIR 2023- 5006 (100%-REGION 4) (STREAM STATS) ft Normal Water Surface Elev 92.2 ft SIR 2023-5006 (HR4) FIELD OBSERVATION Q₁₀= 191 (11.8) = 889~ 890cfs Nithout Q₂₅= 275 (11.8) = 1255 ~1260cfs N/A ft River Station N/A **DESIGN DATA** $Q_{50} = 355 (11.8) = 1600 \sim 1600 cfs$ Q₁₀₀= 437 (11.8) = 1945~ 1950cfs $Q_{roo} = 646 (11.8) = 2819 \sim 2820 cfs$ HEC RAS 6.1.0 (PROJECT: 250208 TurnbullCreek SR2046) *Design Tailwater: Q₁₀ 6.9 ft Q₂₅ 7.3 ft Q₅₀ N/A ft Q₁₀₀ 7.8 ft Q₅₀₀ 8.3 ft *TAILWATER DEPTHS BASED ON WSEL AT RS1100 (DS TOE MINUS INVERT AT OUTLET) STATION 1192 APPROXIMATELY 21 FEET UPSTREAM OF CULVERT OUTLET CONTROL REMARKS ΗW WSEL (BASED ON 8.42' HT OF PIPE) 6.8 93.5' OUTLET CTRL(HW/D-0.81) 7.4 94.1' OUTLET CTRL(HW/D-0.88) OUTLET CTRL(HW/D-1.05) 8.8 95.5' 9.5 96.2' OUTLET CTRL(HW/D-1.13) sf fps Natural Channel Velocity 2.2 (10YR) fps STA. 14+27 -L-3@14'-0"x 8'-5"; ALUMINUM STRUCTUAL PLATE PIPE ARCH WITH HEADWALL AND ENDWALL NCDOT PERFORMANCE TABLE SKEW-90 ELEVATION- 97.4' 100' 50-YEAR | 100-YEAR | 500-YEAR 10-YEAR 25-YEAR NATURAL 93.3' 93.6' N/A 94.2' 94.8' EXISTING 3@ 142"x 91" CMPA TO BE REMOVED EXISTING EXISTING 95.8' 96.4' 93.6' 94.4' N/A PROPOSED 93.5' 94.1' N/A 95.5' 96.2' SECTION 1192 21 FEET FROM UPSTREAM FACE OF CULVERT EXISTING NG RIGHT EXISTING STREAM BED PROFILE PIPE SLOPE- 0.008 FT/FT NOTES: CLINVERT-86.5 1)HEC RAS MODEL RESULTS AND FIELD VERIFICATION INDICATE NO STRUCTURES WILL BE ADVERSELY AFFECTED BY THE WATER SURFACE ELEVATIONS FROM THIS PROJECT. 2)SEE ROADWAY PLANS FOR PROPOSED R/W AND EASEMENTS 0602 **RAYFIELD HAMMONDS** 3@14'-0"x_8'-5"; ALUMINU DWALL AND ENDWALL RUCTUAL PLATE PIPE ARCH WITH EXISTING 50'SONC HW TURNBUI CREEK REMOVE EXISTING 3@ 142"x 91" CMPA AND PROPOSED PIPE FROM TEMPORARY 0503 BULL CREEK FARMS LLC

0601 06014



CULVERT -L- 14+27.00 - SUPPORTING CALCULATIONS





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07/02/2024

Replace Structure	No. 2	208 on	SR 2046	(Avery	Road)
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JOB NAME	over Turnbull Creek	BP6.R021	COMPUTED BY	Kelly de Montbrun, P.E.	_
SUBJECT	Culvert Recommendations Calculations		CHECKED BY	Michael Walko, P.E.	

Structure on SR 2046 (Avery Road) over Turnbull Creek at -L- Station 14+27.00

PROVIDED INFORMATION	
Station	14+27.00 -L-
Structure Type	3 @ 14-ft x 8-ft 5-in Aluminum Structural Plate Pipe
Invert Elevation	@ CL of -L- 86.50 ft
Length	60.0 ft
Width	32.0 ft
Slope	0.008 ft/ft
ESTIMATED INFORMATION	
Foundation Conditioning Material Thickness	Assumed 1.0 ft
Bottom of Culvert Elevation along Culvert Centerline	86.7 ft (LT)
(looking upstation)	86.5 ft (CT)
	86.2 ft (RT)
Bottom of Excavation along Culvert Centerline	= 85.7 ft (LT)
(looking upstation)	
	= 85.5 ft (CT)
	= 85.5 ft (CT) = 85.2 ft (RT)

We anticipate culvert excavation will be within roadway embankment and alluvial materials. The soils generally consisted of the following:

Roadway embankment - Loose to medium dense, silty sand (A-2-4), with trace gravel, asphalt, and organics throughout. **Alluvial** – Very soft to stiff, clayey silt (A-5) and very loose to medium dense silty fine to coarse sand (A-2-4), with trace pebbles, gravel, and organics throughout.

Groundwater was encountered between approximate elevations 91.7 and 91.9 ft. We anticipate groundwater to impact construction.

We do not anticipate WR and CR to impact construction based on the prepared (enclosed) Structure Subsurface Investigation Report.

Based on Borings B-01 and B-02, the culvert will be founded on very loose to medium dense silty sand. The proposed structure is generally within the existing structure footprint with minimal fill anticipated (< 1 ft). We do not anticipate settlement to be a concern.

ESTIMATED QUANTITIES

Foundation Condition Material (Class V or VI) – Backfilling in Water, assume up to 1.0 ft of FCM is placed.

Total Estimated Volume of FCM: 60.0 ft (length) * 32.0 ft (outside width of undercut excavation) * 1.0 ft (undercut) = 1,920.0 ft³ / (27 ft³/yd³) = 71.1 yd³, say 80 yd³

Total Weight of FCM: 71.1 yd³ * 1.485 tons/yd³ = 105.6 tons, say 110 tons

